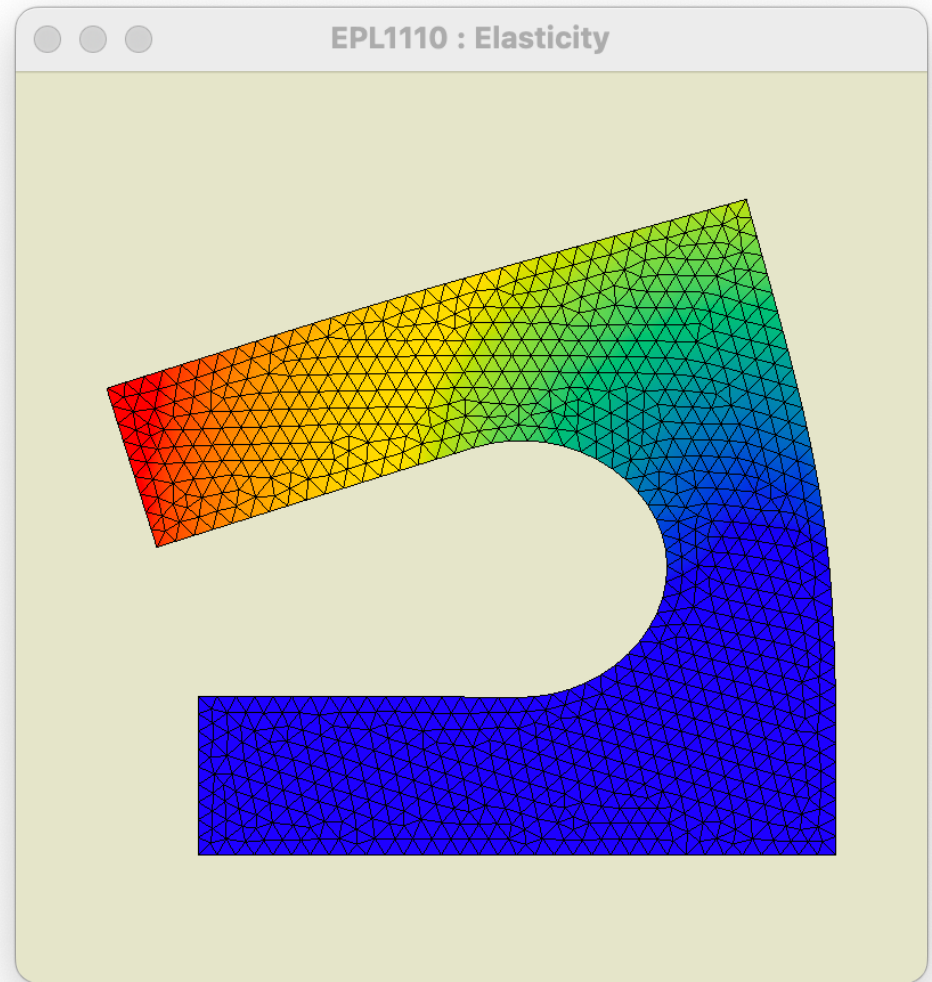
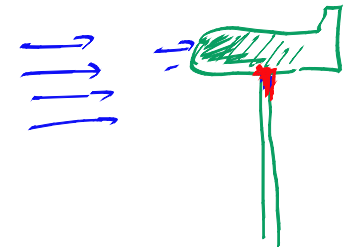
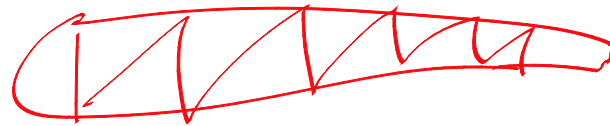
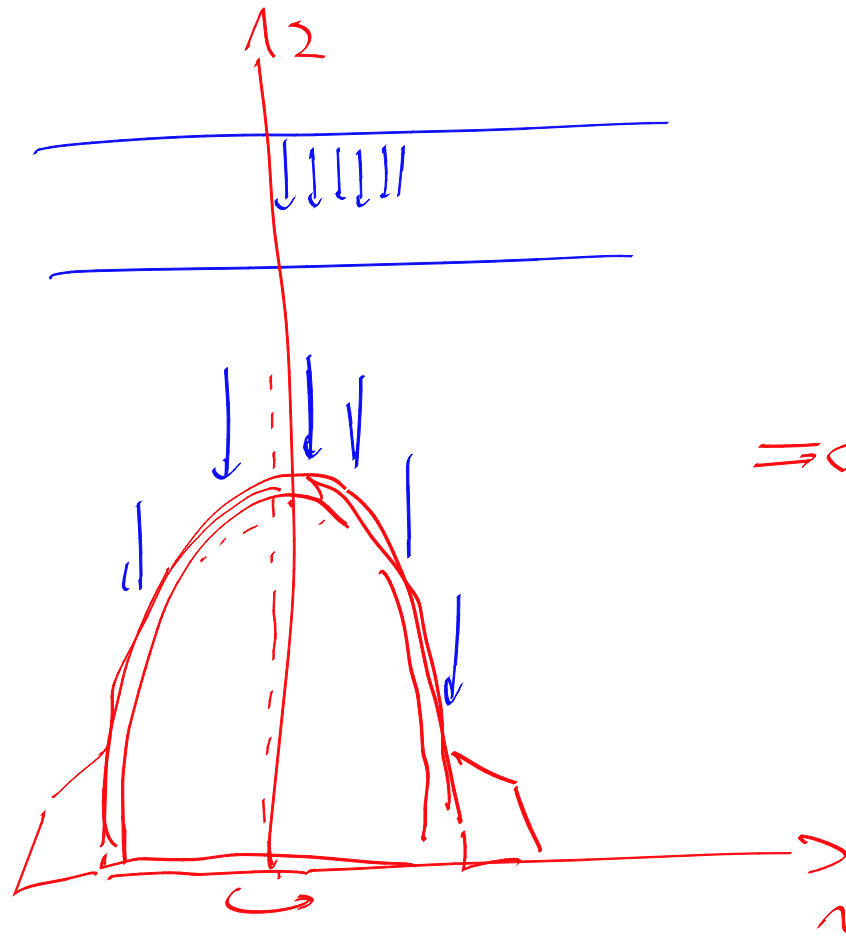


Projet 2023-24



Questions ?



**Ecrire un code informatique efficace
pour l'élasticité linéaire plane**

Tensions planes et déformations planes

Triangles linéaires ou quadratiques

Quads bilinéaires ou biquadratiques

Problèmes axisymétriques

Conditions essentielles en xy et en normale/tangentielle

Conditions naturelles en xy et en normale/tangentielle

Obtenir les tensions dans le domaine

**2 parties
dans le projet !**

Définir un problème original !

Le résoudre avec votre code !

Analyser le résultat !

Projet 2023-24



mesh.txt

```
Number of nodes 335
0 : 0.0000000e+00 1.0000000e+00
1 : 0.0000000e+00 0.0000000e+00
2 : 1.0000000e+00 1.0000000e+00
3 : 1.0000000e+00 7.5000000e-01
4 : 5.0000000e-01 7.5000000e-01
5 : 5.0000000e-01 2.5000000e-01
6 : 1.0000000e+00 2.5000000e-01
7 : 1.0000000e+00 0.0000000e+00
8 : 0.0000000e+00 9.5000000e-01
9 : 0.0000000e+00 9.0000000e-01
```



problem.txt

```
Type of problem : planar strains
Young modulus   : 2.1100000e+11
Poisson ratio   : 3.0000000e-01
Mass density    : 7.8500000e+03
Gravity         : 9.8100000e+00
```



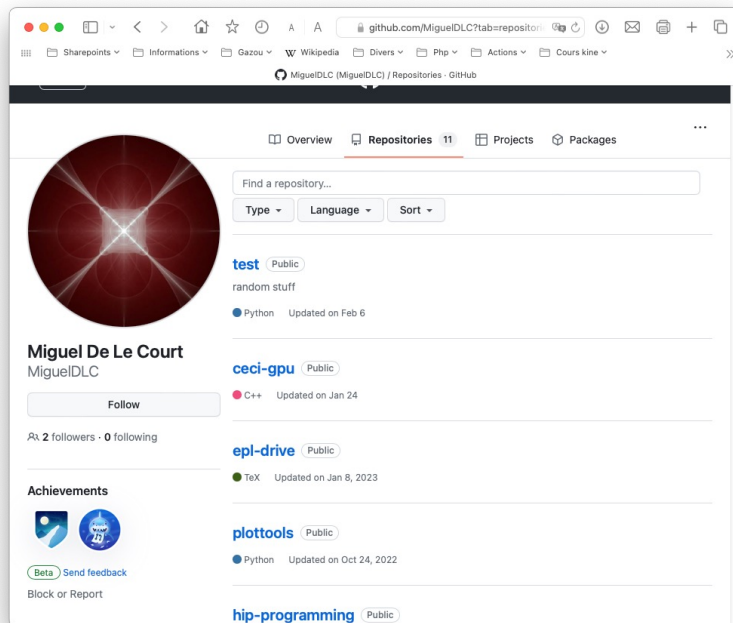
myFem



result.txt

```
Elastic deformation 335
0 : 0.0002330e-03 1.0000000e+00
1 : 0.0000000e+00 0.0000000e+00
```


Tous les trucs utiles ! La semaine prochaine avec Miguel



Si !

```
for (iElem = 0; iElem < theMesh->nElem; iElem++) {
    for (j=0; j < nLocal; j++) {
        map[j] = theMesh->elem[iElem*nLocal+j];
        mapX[j] = 2*map[j];
        mapY[j] = 2*map[j] + 1;
        x[j] = theNodes->X[map[j]];
        y[j] = theNodes->Y[map[j]];

    for (iInteg=0; iInteg < theRule->n; iInteg++) {
        double xsi = theRule->xsi[iInteg];
        double eta = theRule->eta[iInteg];
        double weight = theRule->weight[iInteg];
        femDiscretePhi2(theSpace,xsi,eta,phi);
        femDiscreteDphi2(theSpace,xsi,eta,dphidxsi,dphideta);

        double dxdxsi = 0.0;
        double dxdeta = 0.0;
        double dydxsi = 0.0;
        double dydeta = 0.0;
        for (i = 0; i < theSpace->n; i++) {
            dxdxsi += x[i]*dphidxsi[i];
            dxdeta += x[i]*dphideta[i];
            dydxsi += y[i]*dphidxsi[i];
            dydeta += y[i]*dphideta[i]; }
        double jac = fabs(dxdxsi * dydeta - dxdeta * dydxsi);

        for (i = 0; i < theSpace->n; i++) {
            dphidx[i] = (dphidxsi[i] * dydeta - dphideta[i] * dydxsi) / jac;
            dphidy[i] = (dphideta[i] * dxdxsi - dphidxsi[i] * dxdeta) / jac; }
        for (i = 0; i < theSpace->n; i++) {
            for(j = 0; j < theSpace->n; j++) {
                A[mapX[i]][mapX[j]] += (dphidx[i] * a * dphidx[j] +
                    dphidy[i] * c * dphidy[j]) * jac * weight;
                A[mapX[i]][mapY[j]] += (dphidx[i] * b * dphidy[j] +
                    dphidy[i] * c * dphidx[j]) * jac * weight;
                A[mapY[i]][mapX[j]] += (dphidy[i] * b * dphidx[j] +
                    dphidx[i] * c * dphidy[j]) * jac * weight;
                A[mapY[i]][mapY[j]] += (dphidy[i] * a * dphidy[j] +
                    dphidx[i] * c * dphidx[j]) * jac * weight; }}

            for (i = 0; i < theSpace->n; i++) {
                B[mapY[i]] -= phi[i] * g * rho * jac * weight; }}}

int *theConstrainedNodes = theProblem->constrainedNodes;
for (int i=0; i < theSystem->size; i++) {
    if (theConstrainedNodes[i] != -1) {
        double value = theProblem->conditions[theConstrainedNodes[i]]->value;
        femFullSystemConstrain(theSystem,i,value); }}

return femFullSystemEliminate(theSystem);
```

Les sponsors du projet ! Ingés en transition

24 avril 2024
Salle Nyquist

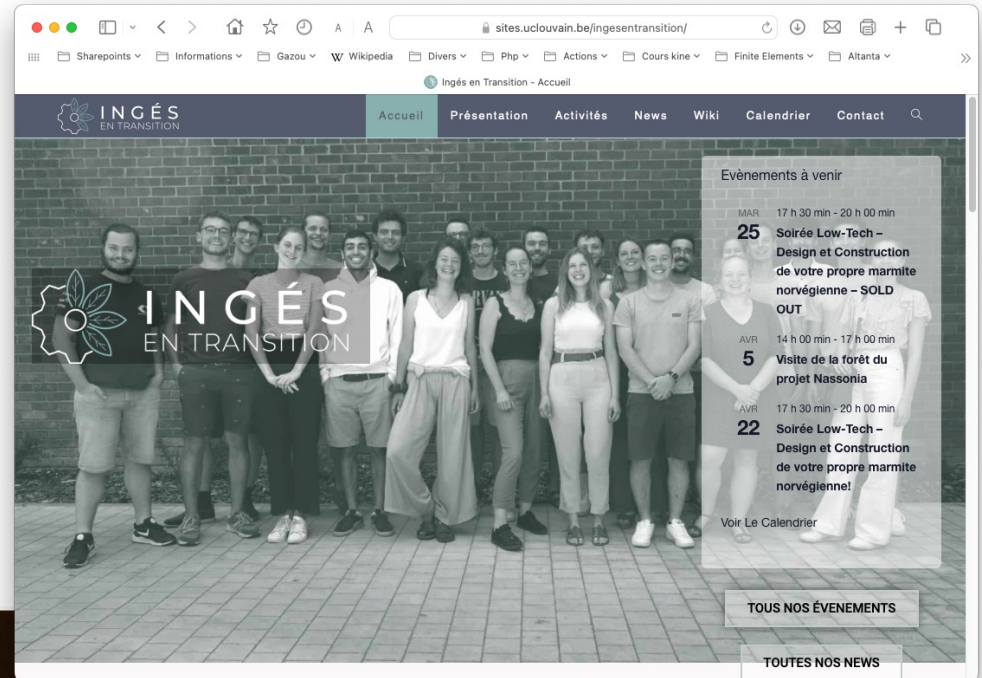
MER
24

24 avril, 18 h 30 min - 20 h 30 min

Table Ronde: Marcher sur du béton pour faire pousser de l'herbe ?

Salle Nyquist (A164) du bâtiment Maxwell Pl. du Levant 3., Ottignies-Louvain-la-Neuve, Belgique

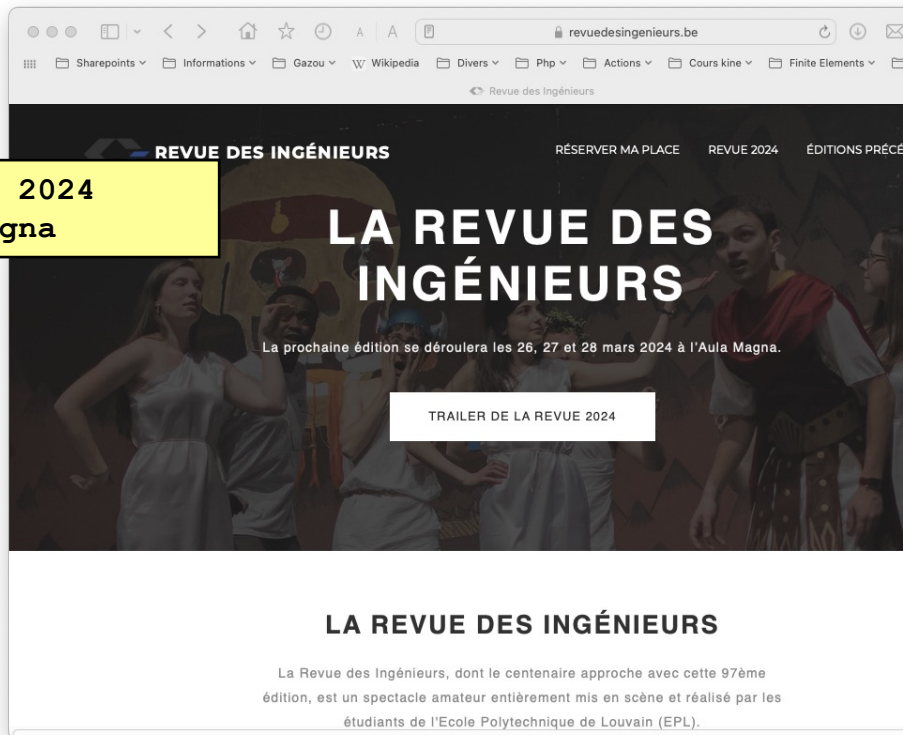
L'activisme comme levier de la transition écologique ? Des marches pour le climat à code rouge, beaucoup sont ceux qui participent de près ou de loin, de façon radicale ou modérée, à l'activisme écologique. Mais est-ce que tous ces efforts valent-ils la peine ? Quelle est la place de l'activisme dans la transition ? Nous [...]



Non, non !
Assister à la Table Ronde ne donne aucun bonus pour le projet !
Bien essayer, Charles. Mais c'est loupé !
Oui : l'activisme dans la transition, c'est un choix politique respectable !

Les sponsors du projet ! La revue des ingénieurs

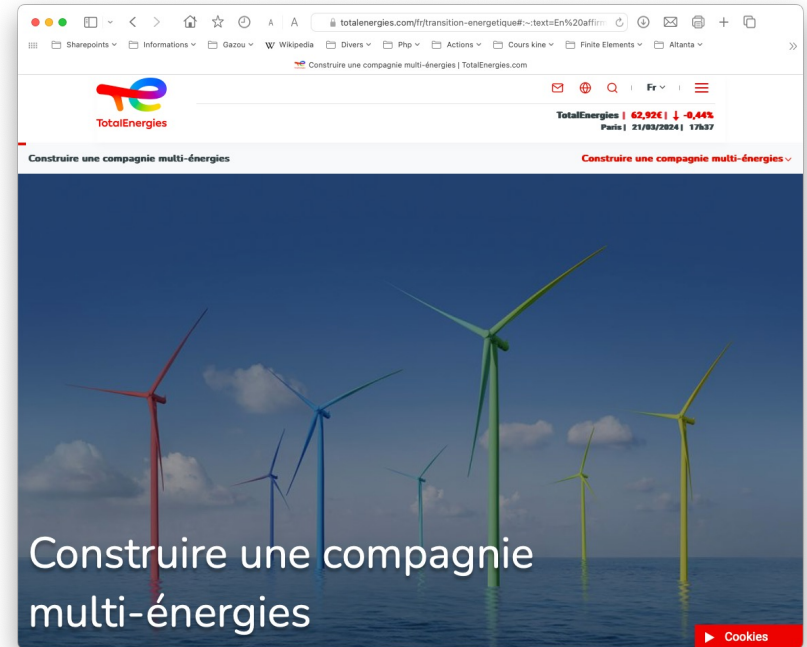
27 mars 2024
Aula Magna



Non, non !
Assister à la Revue ne donne aucun bonus pour le projet !
Bien essayer, Charles. Mais c'est loupé !
Oui : c'est un joli spectacle mais pas de Youpidou cette année encore :-)

Les sponsors du projet ! TotalEnergies

Victoire pour les organisations environnementales : les 20km de Bruxelles mettent fin à leur partenariat avec TotalEnergies



En affirmant son ambition d'être un acteur majeur de la transition énergétique et d'atteindre la neutralité carbone à horizon 2050, ensemble avec la société, TotalEnergies s'engage à faire évoluer en profondeur ses productions et ses ventes, tout en continuant à répondre aux besoins en énergie des populations en croissance.

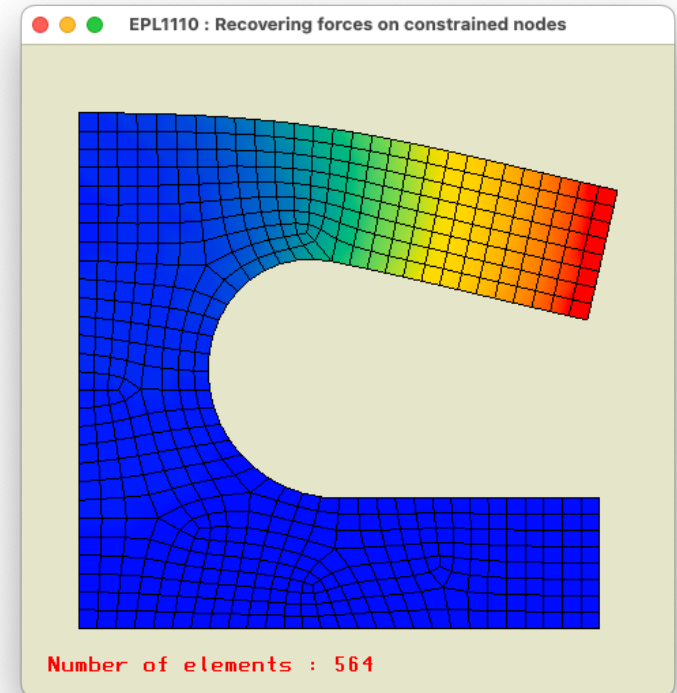
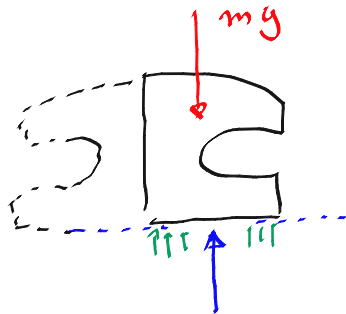
Le prochain devoir !

Appliquer des conditions de Neumann !

Retrouver les forces de réactions !

Intégrer la densité de force de gravité !

Vérifier la seconde et la troisième loi de Newton !



```
void femElasticityAssembleElements(femProblem *theProblem)
void femElasticityAssembleEdges(femProblem *theProblem)
void femElasticitySolve(femProblem *theProblem)
void femElasticityForces(femProblem *theProblem)
void femElasticityIntegrate(femProblem *theProblem, double (*f)(double,double))
```

Conditions aux limites

Linear elasticity problem

Young modulus $E = 2.1100000e+11$ [N/m²]

Poisson's ratio $\nu = 3.0000000e-01$ [-]

Density $\rho = 7.8500000e+03$ [kg/m³]

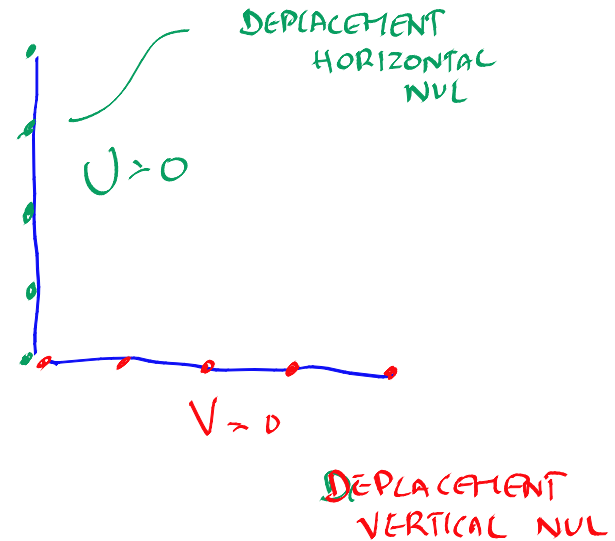
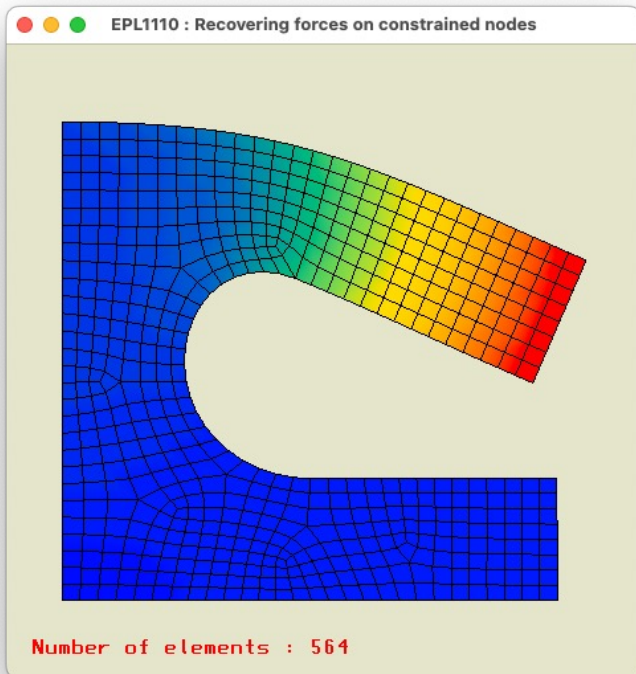
Gravity $g = 9.8100000e+00$ [m/s²]

Planar strains formulation

Boundary conditions :

Symmetry : imposing $0.00e+00$ as the horizontal displacement

Bottom : imposing $0.00e+00$ as the vertical displacement



Forces appliquées sur les nœuds contraints

Linear elasticity problem

Boundary conditions :

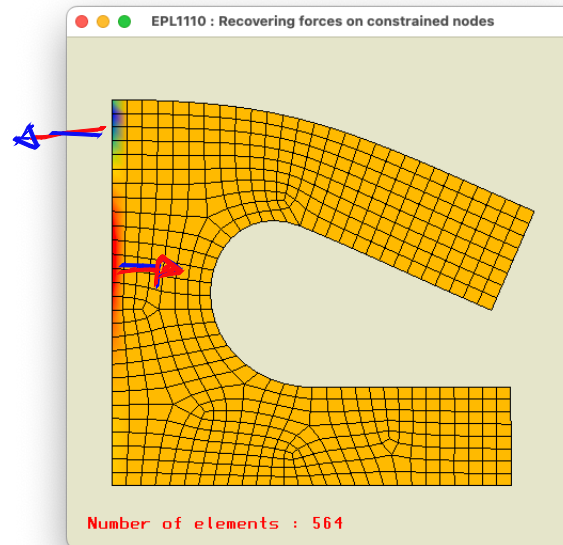
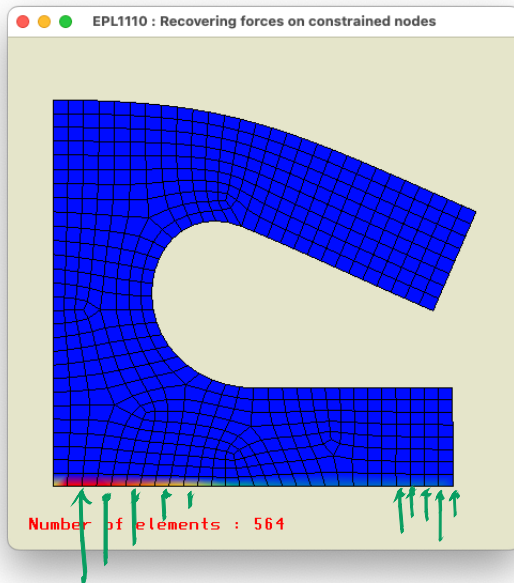
Symmetry : imposing $0.00e+00$ as the horizontal displacement

Bottom : imposing $0.00e+00$ as the vertical displacement

Top : imposing $-1.00e+04$ as the vertical force density

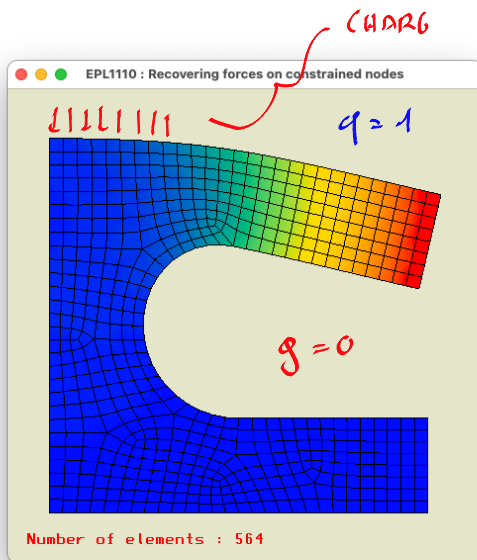
```
==== Minimum displacement      : 0.0000000e+00 [m]
==== Maximum displacement     : 1.6227532e-06 [m]
==== Global horizontal force   : 2.0317257e-10 [N]
==== Global vertical force     : 1.0000000e+04 [N]
==== Weight                    : 0.0000000e+00 [N]
```

RESIDUS
EQUILIBRE EN Y

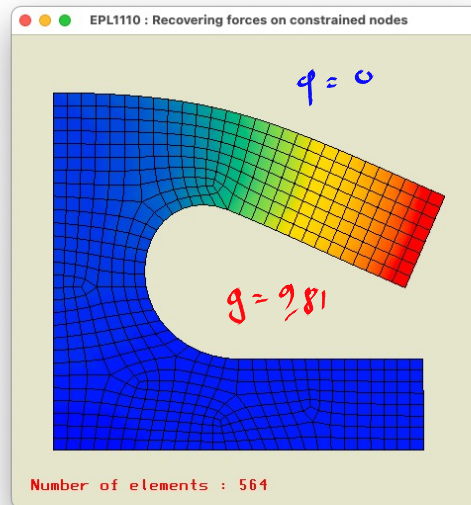


RESIDUS
EQUILIBRE EN X

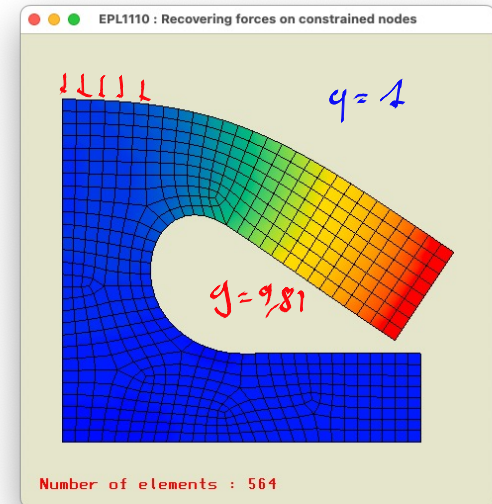
Modifions le problème



Déformation
sous
la charge



Déformation
sous son poids propre



Déformation
sous la charge et son
poids propre

```
double E = 211.e9;
double nu = 0.3;
double rho = 7.85e3;
double g = 0.00;
femProblem* theProblem = femElasticityCreate(theGeometry,E,nu,rho,g,PLANAR_STRAIN);
femElasticityAddBoundaryCondition(theProblem,"Symmetry",DIRICHLET_X,0.0);
femElasticityAddBoundaryCondition(theProblem,"Bottom",DIRICHLET_Y,0.0);
femElasticityAddBoundaryCondition(theProblem,"Top",NEUMANN_Y,-1e4);
femElasticityPrint(theProblem);
```

Modifions le problème

Linear elasticity problem

Young modulus $E = 2.1100000e+11$ [N/m²]

Poisson's ratio $\nu = 3.0000000e-01$ [-]

Density $\rho = 7.8500000e+03$ [kg/m³]

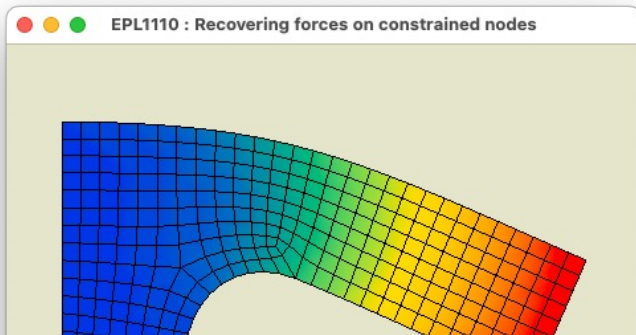
Gravity $g = 9.8100000e+00$ [m/s²]

Planar strains formulation

Boundary conditions :

Symmetry : imposing $0.00e+00$ as the horizontal displacement

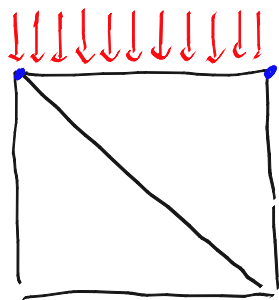
Bottom : imposing $0.00e+00$ as the vertical displacement



```
double E = 211.e9;
double nu = 0.3;
double rho = 7.85e3;
double g = 0.00;
femProblem* theProblem = femElasticityCreate(theGeometry,E,nu,rho,g,PLANAR_STRAIN);
femElasticityAddBoundaryCondition(theProblem,"Symmetry",DIRICHLET_X,0.0);
femElasticityAddBoundaryCondition(theProblem,"Bottom",DIRICHLET_Y,0.0);
femElasticityAddBoundaryCondition(theProblem,"Top",NEUMANN_Y,-1e4);
femElasticityPrint(theProblem);
```

Conditions de Neumann !

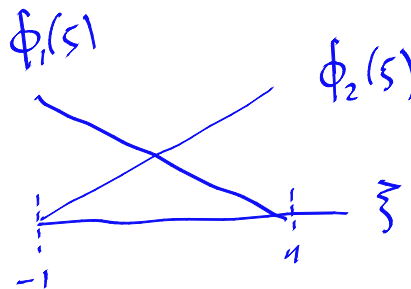
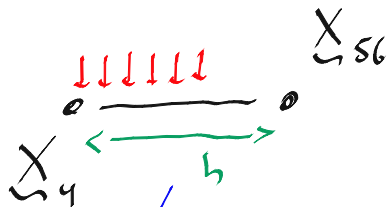
$$g = \begin{bmatrix} g_x \\ g_y \end{bmatrix} = \begin{bmatrix} -10^4 \\ 0 \end{bmatrix}$$



$$\sum_j \langle \nabla_{\xi} \tau_i \cdot \nabla_{\xi} \tau_j \rangle U_j = \langle g \tau_i \rangle_N$$

$$\left\{ \begin{array}{l} \langle g_x \tau_i \rangle_N \\ \langle g_y \tau_i \rangle_N \end{array} \right.$$

ASSOCIEES AUX EQUATIONS DES DEPLACEMENTS EN ξ



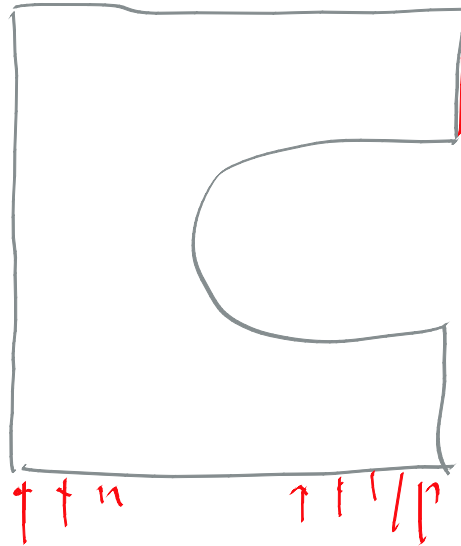
$$\begin{aligned} \phi_1 &= (1-\xi)/2 \\ \phi_2 &= (1+\xi)/2 \end{aligned}$$

$$J = \frac{h}{2}$$

$$h = \sqrt{(X_4 - X_{S6})^2 + (Y_4 - Y_{S6})^2}$$

Les conditions aux limites d'un problème d'élasticité linéaire.

$$\begin{aligned} \Delta^2 u &= f = 0 \\ u &= \bar{u} \quad \text{SUR } \Gamma_D \\ \eta \cdot \nabla u &= \bar{g} \quad \text{SUR } \Gamma_N \end{aligned}$$



ESSENTIELLE
DIRICHLET

IL FAUT
SOIT
IMPOSER
LE DEPLACEMENT

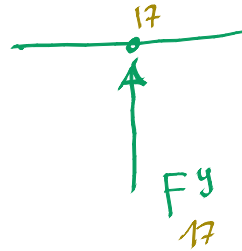
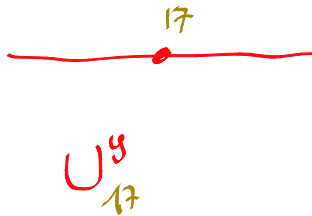
SOIT
IMPOSER
UNE DENSITE
DE FORCE

NATURELLE
NEUMANN

On peut appliquer une condition de Dirichlet ou...
une condition de Neumann équivalente.

$$U_{17}^y = V_{17} = 0$$

CONDITION
ESSENTIELLE



$$\ll g_y \tau_{17} \gg$$

CONDITION
NATURELLE
EQUIVALENTE

Retrouver les
forces de contact !

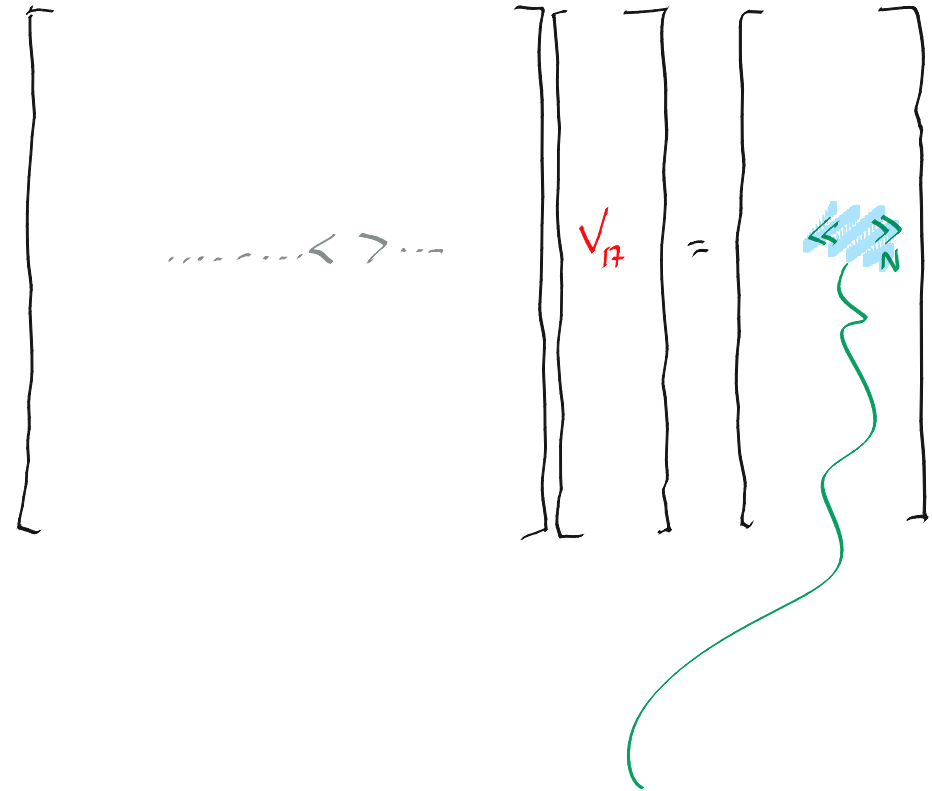
$$\delta s = -17 \times 2 + 1 \dots$$

$$U_{17}^y = V_{17} = 0$$

CONDITION
ESSENTIELLE



$$U_{17}^y$$

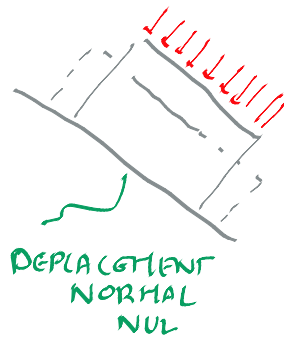
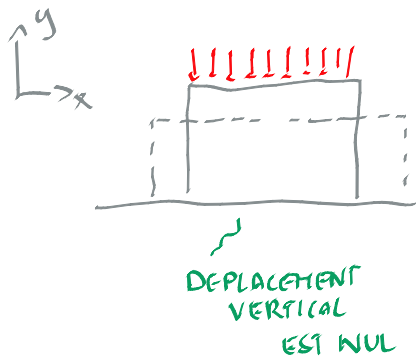


$$\langle g_y \tau_{17} \rangle$$

$$F_{17}^y$$

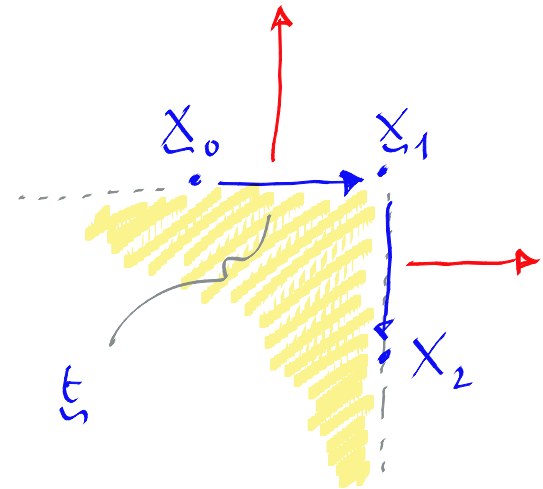
Exprimer les conditions aux limites en termes de normale et tangente

$$\mathbf{n} \cdot \mathbf{t} = 0$$

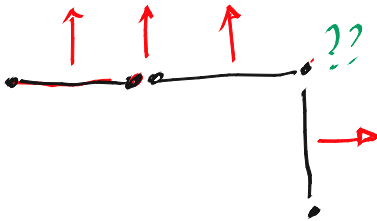


$$\mathbf{u} = \begin{bmatrix} u \\ v \end{bmatrix}$$

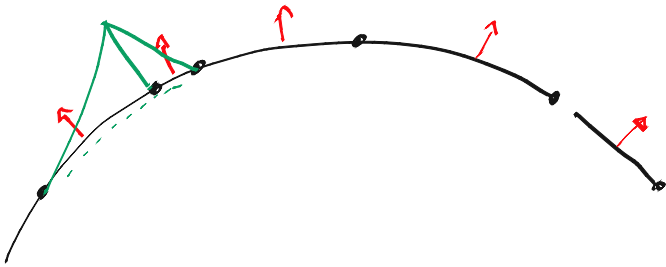
$$\mathbf{n} = \begin{bmatrix} n_x \\ n_y \end{bmatrix} = \begin{bmatrix} -t_y \\ t_x \end{bmatrix}$$



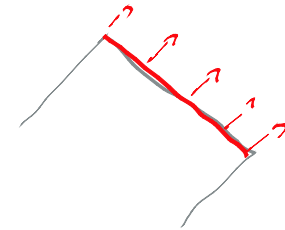
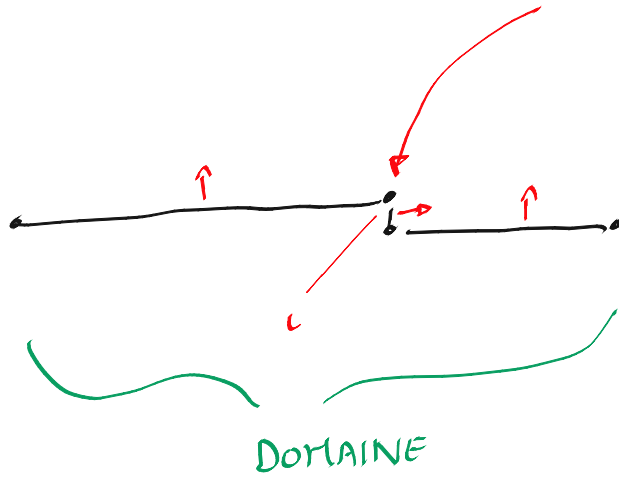
$$\mathbf{t} = \begin{bmatrix} \Delta x \\ \Delta y \end{bmatrix} = \mathbf{x}_1 - \mathbf{x}_0$$

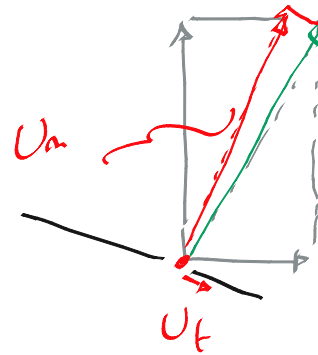
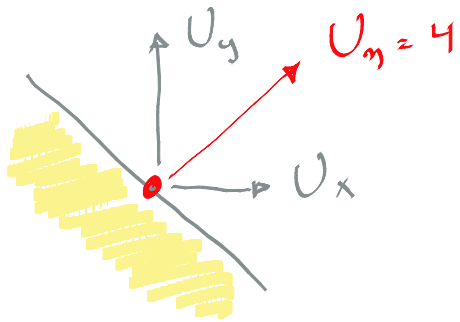


$$m = \sqrt{n_x^2 + n_y^2}$$



$$N_i = \langle \phi_i, n \rangle$$





$$U_m = \alpha U_x + \beta U_y$$

$$U_t = \gamma U_x + \delta U_y$$

$$\begin{bmatrix} \times & \times & \times & \times & \times & \times & \times \\ \times & \times & \times & \times & \times & \times & \times \end{bmatrix} \begin{bmatrix} U_x \\ U_y \end{bmatrix} = \begin{bmatrix} \times & \times \\ \times & \times \\ \times & \times \\ \times & \times \end{bmatrix}$$

$$m_x U_x + m_y U_y = 4$$

CELI
EST LA
CONDITION
ESSENTIELLE
QUE JE
VEUX IMPOSER

$$\begin{bmatrix} U_m \\ U_t \end{bmatrix}$$

$$J(u) = \frac{1}{2} \langle \nabla_x u, \nabla_x u \rangle - \langle f, u \rangle$$

$$u \approx u_h = \sum_{i=1}^m U_i \tau_i$$

$$J(u_h) = \frac{1}{2} \langle (\sum_i \nabla_{\tau_i} U_i) (\sum_j \nabla_{\tau_j} U_j) \rangle - \langle f (\sum U_i \tau_i) \rangle$$

$$A_{ij} = \langle \nabla_{\tau_i}, \nabla_{\tau_j} \rangle$$

$$B_i = \langle f, \tau_i \rangle$$

$$= \frac{1}{2} \sum_i \sum_j A_{ij} U_j - \sum_i B_i U_i$$

$$\sum_j A_{ij} U_j = B_i$$

$$U_n = a + b U_1$$

$$U_n = a + b U_1$$

$$U_h = \sum_{i=1}^{n-1} \tau_i U_i + \tau_n (a + b U_1)$$

$$J(U_h) = \frac{1}{2} \left\langle \left(\sum_i^m \nabla_{\tau_i} U_i \right) \left(\sum_j^m \nabla_{\tau_j} U_j \right) \right\rangle - \left\langle f \left(\sum_i^m U_i \tau_i \right) \right\rangle$$

$$\begin{aligned}
 J(U_h) = & \frac{1}{2} \sum_{i=1}^{n-1} \sum_{j=1}^{n-1} A_{ij} U_i U_j - \sum_{i=1}^{n-1} B_i U_i \\
 & + \sum_{i=1}^{n-1} A_{i,n} U_i (b + a U_1) \\
 & + \frac{1}{2} A_{n,n} (b + a U_1)(b + a U_1) - B_n (b + a U_1)
 \end{aligned}$$

$$0 = \frac{\partial J}{\partial U_i}$$

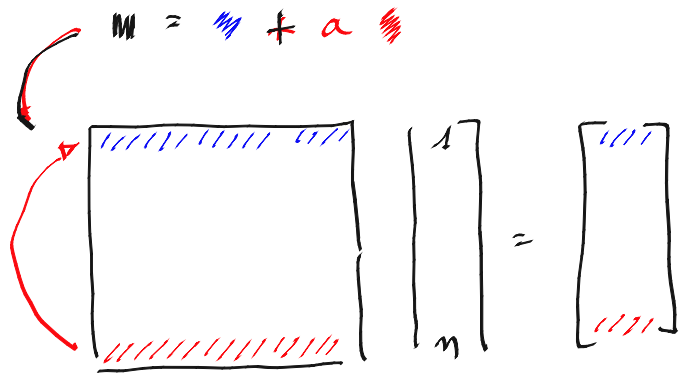
$i = 1, \dots, n+1$

EQUATION
POUR 1

$$0 = a \left(\sum A_{nj} U_j + A_{n,n} (a + b U_1) \right) - B_n + \sum A_{1j} U_j - B_1$$

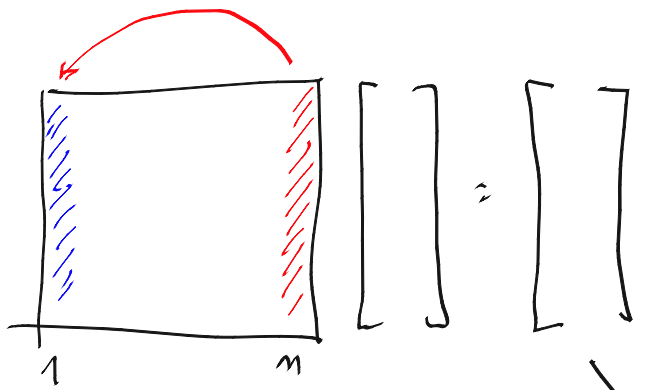
$$\sum_{j=1}^{n-1} A_{ij} U_j - B_i + A_{i,n} (b + a U_1) = 0$$

EQUATIONS
2 ... n-1



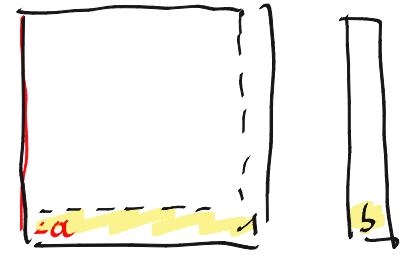
$U_n = b + a U_1$

$L1 \leftarrow L_n a + L1$



$C1 \leftarrow C_n a + C1$

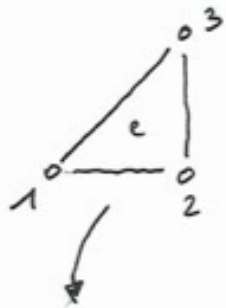
$M = \text{blue} + a \text{ red}$



$M = \square - b \text{ red}$

Et l'axisymétrique ?

COMPUTING A LOCAL ELASTICITY MATRIX



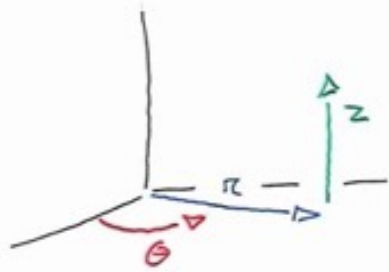
	$\tau_{i,x}$	$\tau_{i,y}$
1	-1	0
2	1	-1
3	0	1

$$\left[\begin{array}{c|c} A \langle \tau_{i,x} \tau_{j,x} \rangle + C \langle \tau_{i,y} \tau_{j,y} \rangle & B \langle \tau_{i,x} \tau_{j,y} \rangle + C \langle \tau_{i,y} \tau_{j,x} \rangle \\ \hline B \langle \tau_{i,y} \tau_{j,x} \rangle + C \langle \tau_{i,x} \tau_{j,y} \rangle & A \langle \tau_{i,y} \tau_{j,y} \rangle + C \langle \tau_{i,x} \tau_{j,x} \rangle \end{array} \right]$$

$$A_{\equiv ij}^e = \begin{cases} A_{xxij} = A \\ A_{yyij} = C \\ A_{xyij} = 0 \\ A_{yxij} = 0 \end{cases}$$

$$A_{\equiv ij}^e = \frac{1}{2} \left[\begin{array}{cc|cc} A & 0 & -A & B \\ 0 & C & C & -C \\ \hline A+C & -B-C & -C & B \\ -B-C & A+C & C & -A \\ \hline & & C & 0 \\ & & 0 & A \end{array} \right]$$

AXISYMMETRIC PROBLEMS



AXISYMMETRY!

$$\begin{aligned} v_\theta &= 0 \\ \frac{\partial}{\partial \theta} &= 0 \end{aligned}$$

$$u = \begin{bmatrix} u_r \\ u_\theta \\ u_z \end{bmatrix} = \begin{bmatrix} u(r, z) \\ 0 \\ v(r, z) \end{bmatrix}$$

$$u_m = \begin{bmatrix} u_{r,r} & 0 & (u_{r,z} + u_{z,r})/2 \\ u_r/r & 0 & 0 \\ 0 & 0 & u_{z,z} \end{bmatrix}$$

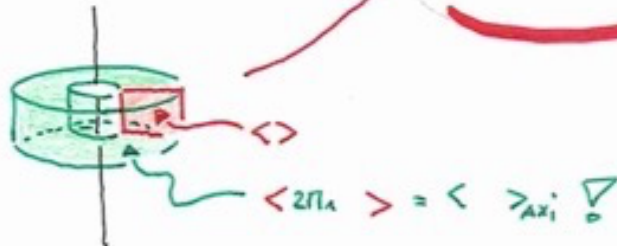
A, B, C
OF PLANAR DEFORMATIONS !

$$e_m = \begin{bmatrix} A \epsilon_{rr} + B (\epsilon_{\theta\theta} + \epsilon_{zz}) & 0 & 2C \epsilon_{rz} \\ A \epsilon_{\theta\theta} + B (\epsilon_{rr} + \epsilon_{zz}) & 0 & 0 \\ A \epsilon_{zz} + B (\epsilon_{rr} + \epsilon_{\theta\theta}) & 0 & 0 \end{bmatrix}$$

$$U(\tau_i, 0, 0) = \begin{bmatrix} \tau_{i2} & 0 & \tau_{i2}/2 \\ 0 & \tau_i/2 & 0 \\ 0 & 0 & 0 \end{bmatrix}$$

$$U(0, 0, \tau_i) = \begin{bmatrix} 0 & 0 & \tau_{i2}/2 \\ 0 & 0 & 0 \\ \tau_{i2} & 0 & 0 \end{bmatrix}$$

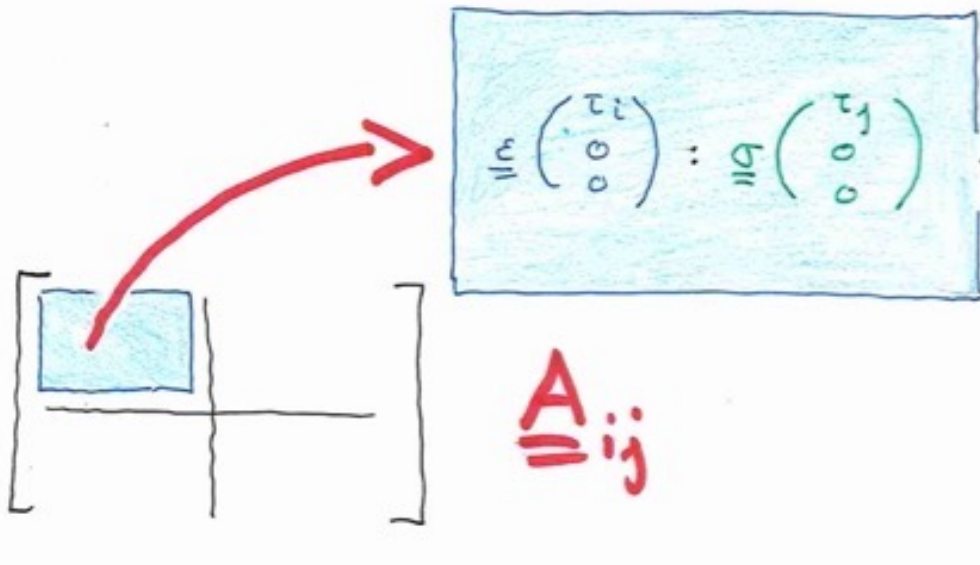
$$2\pi \left[\begin{array}{c|c} \langle \tau_i | U(\tau_i, 0, 0) | \tau_i \rangle & \langle \tau_i | U(0, 0, \tau_i) | \tau_i \rangle \\ \hline \langle \tau_i | U(0, 0, \tau_i) | \tau_i \rangle & \langle \tau_i | U(\tau_i, 0, 0) | \tau_i \rangle \end{array} \right]$$



توی A

$$\|m\| \begin{pmatrix} \tau_i \\ 0 \\ 0 \end{pmatrix} = \begin{bmatrix} \tau_{i,z} & 0 & \tau_{i,z}/2 \\ & \tau_{i,r} & 0 \\ & & 0 \end{bmatrix}$$

$$\|q\| \begin{pmatrix} \tau_j \\ 0 \\ 0 \end{pmatrix} = \begin{bmatrix} A\tau_{j,r} + B\tau_{j,r} & 0 & C\tau_{j,z} \\ & A\tau_{j,r} + B\tau_{j,r} & 0 \\ & & B\tau_{j,r} + B\tau_{j,r} \end{bmatrix}$$



$\|m\| \begin{pmatrix} \tau_i \\ 0 \\ 0 \end{pmatrix} \cdot \|q\| \begin{pmatrix} \tau_j \\ 0 \\ 0 \end{pmatrix} = \langle \tau_{i,r} (A\tau_{j,r} + B\tau_{j,r}) \rangle + \langle \tau_{i,z} C\tau_{j,z} \rangle + \langle \frac{\tau_i}{r} (A\frac{\tau_j}{r} + B\tau_{j,r}) \rangle$

\underline{A}_{ij}

IT IS ALMOST
LIKE A 2D PROBLEM !

$$A \langle \tau_{i,n} \tau_{j,n} \rangle$$

$$+ C \langle \tau_{i,z} \tau_{j,z} \rangle$$

$$+ B \langle \tau_{i,n} \tau_j \rangle$$

$$+ \langle \tau_i (B \tau_{j,n} + A \frac{\tau_j}{2}) \rangle$$

ϵ_{00}/λ

σ_{00}

$$B \langle \tau_{i,z} \tau_{j,n} \rangle$$

$$+ C \langle \tau_{i,n} \tau_{j,z} \rangle$$

$$+ B \langle \tau_{i,z} \tau_j \rangle$$

$$B \langle \tau_{i,n} \tau_{j,z} \rangle$$

$$+ C \langle \tau_{i,z} \tau_{j,n} \rangle$$

$$+ B \langle \tau_i \tau_{j,z} \rangle$$

$$A \langle \tau_{i,z} \tau_{j,z} \rangle$$

$$+ C \langle \tau_{i,n} \tau_{j,n} \rangle$$